

**DESIGN CALCULATION AND DRAWING FOR
LONGITUDINAL GUIDED BEARING**

(Ch. 516+938)

PROJECT: FOUR LANING OF JHANJHI TO DEMOW SECTION OF NH-37 FROM EXISTING CH. K 491+050 TO KM 535+250
(DESIGN CH. KM 4900+800 TO KM 534+800) IN THE STATE
OF ASSAM UNDER EPC MOD

CLIENT: NATIONAL HIGHWAYS & INFRASTRUCTURE DEVELOPMENT
CORPORATION LTD. (NHIDCL)

CONTRACTOR : M/S KAMAC-SHIVA HARLALKA (JV)

MANUFACTURER:



M/S KARMA ENTERPRISE, GUWAHATI, ASSAM

Design Calculation of SLS- 1647 KN, ULS- 2365 KN Longitudinal Guided Bearing

TYPE OF BEARING : LG TYPE: B3 Conc. Grade for Pedestal M 45
 REVISION : 00 CH: 516+938 Conc. Grade for Superstructure M 50

DESIGN DATA FOR BEARING DESIGN (LOADS)

LOADING PARAMETERS	SLS		ULS	
UNITS	MT	kN	MT	kN
VERTICAL MAXIMUM LOAD	167.94	1647.00	241.16	2365.00
VERTICAL MINIMUM LOAD	68.63	673.00	117.27	1150.00
VERTICAL PERMANENT LOAD	85.96	843.00		
HORIZONTAL LOADS				
ACTING IN LONGITUDINAL DIRECTION	0.00	0.00	0.00	0.00
ACTING IN TRANSVERSE DIRECTION	0.00	0.00	0.00	0.00
RESULTANT HORIZONTAL LOADS		0.00		0.00
DISPLACEMENT				
LONGITUDINAL (MM)	+ 21.00	-21.00		42.00
TRANSVERSE (MM)	0.00	0.00		0.00
ROTATION (RADIAN)	0.0038	+ 0	=	0.004

Assume Permanent Rotation q p = 0.41 x 0.004 = 0.002
 Assume Variable Rotation q v = 0.59 x 0.004 = 0.002

MATERIALS

Steel Stress (Working) for Design Use 340 MPa (Grade-340-570W) AS PER IS - 1030
 HT Bolts shall conform to Grade 8.8 of IS : 1364
 Elastomer shall be of hardness 50 + / - 5 conforming to IRC : 83 (Part III) - 2018 Table - 4.3

DIMENSION DETAILS OF BEARING COMPONENTS

ELASTOMERIC PAD

PAD DIAMETER = di 325 mm
 PAD THICKNESS = he 24 mm

PTFE

PTFE DIAMETER = Dptfe 325 mm
 PTFE THICKNESS = Tptfe 5 mm

CYLINDER

CYLINDER CONCRETE CONTACT DIAMETER = Do 395 mm
 CYLINDER BASE THICKNESS = kb 20 mm
 CYLINDER INNER DIAMETER = Di = di 325 mm
 CYLINDER OUTER DIAMETER = do 375 mm
 HEIGHT OF CYLINDER = hc 36 mm
 WALL THICKNESS = Tew 25.0 mm

INTERMEDIATE COMPONENT

LENGTH = Lic 365 mm
 WIDTH = Wic 365 mm
 INTERMEDIATE COMPONENT ABOVE THICKNESS = Tp 12 mm
 INTERMEDIATE COMPONENT PROJECTION = hp 22 mm
 VERTICAL FACE = w 6 mm

TOP PLATE

EFFECTIVE CONCRETE CONTACT DIAMETER = Dtpeff 375 mm
 LENGTH = Lt 420 mm
 WIDTH = Bt 410 mm
 THICKNESS = Tt 22 mm
 S/S SHEET LENGTH = Lss 410 mm
 S/S SHEET WIDTH = Bss 370 mm
 S/S SHEET THICKNESS = Tss 3 mm
 GUIDE BAR LENGTH = Lgb 420 mm
 GUIDE BAR WIDTH = ku 15 mm
 GUIDE BAR HEIGHT = Hgb 18 mm

Design Calculation of SLS- 1647 KN, ULS- 2365 KN Longitudinal Guided Bearing

ANCHORAGE

BOLTS DIAMETER	=	Dbolt	16 mm
BOLTS LENGTH	=	Lbolt	35 mm
BOLTS PER COMPONENT	=	Nbolt	4 NOS.
GRADE OF BOLTS	=	GR.bolt	8.8
ANCHORAGE COLLOR LENGTH	=	CL	56 mm
ANCHORAGE COLLOR THICKNESS	=	C thk	12 mm
SLEEVE LENGTH (Superstructure)	=	Ls	110 mm
SLEEVE DIAMETER (Superstructure)	=	Ds	40 mm
SLEEVE LENGTH (Pedestal)	=	Lp	120 mm
SLEEVE DIAMETER (Pedestal)	=	Dp	40 mm

GENERAL

NO. OF BRASS SEALING RINGS	=	Nbr	2 NOS.
TOTAL THICKNESS OF RINGS	=	Tbr	4.0 mm
GAP BETWEEN CYLINDER & TOP COMPONENT	=	h4	10 mm
TOTAL BEARING ASSEMBLY HEIGHT	=	HT	105.2 mm

Calculation for Permissible Stresses in Pedestal Concrete

Bottom

Cylinder Concrete Contact Diameter =		395	mm
Loaded area ($A_{co} = p \times D_o \wedge 2/4$) =		122541.75	mm ²
Required Pedestal Size for Dispersion =	790.00	x	790 mm
Dispersed area ($A_{cl} = p \times d \wedge 2/4$) =		490167	mm ²

Top

Top Component Contact Diameter =		375	mm
Loaded area ($A_2 = p \times D_{teff} \wedge 2/4$) =		110446.62	mm ²
Required Superstructure Size for Dispersion =	750.00	x	750 mm
Dispersed area ($A_{c1} = p \times d \wedge 2/4$) =		441786.47	mm ²

DESIGN CALCULATIONS :-

REF. CODE : IRC:83 (Part-III)-2018

Design of Pad (Clause - 5.2.3.2)

Effective diameter of Pad	Dpad		325 mm
Area of pad = $p \times d \wedge 2/4$	a		82991.0714 mm ²
Vertical Load	Nsd	2365.00	kN
Direct Pressure Nsd / a	pa	28.50	N/mm ²
(Nsd / a) x γ_m		37.05	N/mm ²
fe,k		60.00	N/mm ²
		OK	

Check Compression at edge of Neoprene Pad (Clause - 5.2.3.4)

Max. Permitted = 15 % of he	3.60	mm	
Desired Rotation	0.00380	radians	
Available Rotation in Radius due to Compression of Pad	0.02215	radians	OK
Diameter / Thickness Ratio	13.54	Maximum (Dpad/he)	15 OK

Check for Min. average Stress (Clause - 5.2.3.3)

Min. average stress = (Nsd min. / a)	8.11	N/mm ²
Permissible Min. average stress =	2.00	N/mm ²
	OK	

Design Calculation of SLS- 1647 KN, ULS- 2365 KN Longitudinal Guided Bearing

Pressure on PTFE

Diameter of PTFE	L	325.00	mm
Area of PTFE	Aco	82957.68	mm ²
Average pressure on PTFE = $N_{max} ULS / A_p$	σ_p	28.51	N/mm ²
Protrusion of PTFE			
$h = 1.75 + L / 1200$		2.02	mm
h provided		2.20	mm
Thickness of PTFE = 2.2h		4.84	mm
Thickness of PTFE provided		5.00	mm
		OK	
Characteristic compressive strength of PTFE	f_{tk}	90.00	N/mm ²
The Characteristic compressive strength is reduced due to exceeding the category temperature above 30°C.			
Max. ambient temperature		43.00	°C
Reduction		26.00	%
Reduction factor	k	0.74	
$\mu_{max} = 1.2 / (10 + \sigma_p)$		0.031	
	0.03	≤	0.031 ≤ 0.08
$e1 = \mu_{max} \times R$		5.06	mm
$e3 = ad \times L/2$		0.62	mm
$e = e1 + e3$		5.68	mm
	L/8	40.63	mm
		OK	
$\lambda = (1 - 0.75 \times \pi \times e / L)$	λ	0.96	
Reduced contact area $A_r = A_{co} \times \lambda$	A_r	79540.77	mm ²
$max, N_{sd} = A_r \times k \times f_{tk} / \gamma_m$	$\gamma_m = 1.4$	3783.87	kN
N_{sd}, ULS		2365.00	kN
		OK	

Stress in bottom concrete

Bottom dispersion width		Do	395
Thickness of Bottom Plate		kb	20
Area of Bottom Dispersion = $p \times d^{2/4}$	a	122591.071	mm ²
Section Modulus = $p \times d^{3/32}$	Z	6052934.152	mm ³
Vertical Load	N_{sd}	2365.00	kN
Horizontal force	V_{sd}	0.00	kN
Moment of resistance due to rotation:			
Rotation due to dead load	θ_p	0.00155	radians
Live Load	θ_v	0.00225	radians
Ratio = d_i / h_e		13.54	
For induced moment	$k1$	1.97	
	$k2$	76.38	
Induced moment due to rotation $Me.d$			
$d_i^3 \times (k1 \times qp + k2 \times qv) / 1000$	$Me.d$	5997.30	kN-mm
Moment of resistance due to HF:			
Horizontal distance	C	162.50	mm
Resultant HF	V_{sd}	0.00	kN
Moment of resistance due to HF:	$Mr.d$	0.00	kN-mm
$0.2 \times C \times V_{sd}$			
Total Movement = $Me.d + Mr.d$	Mt	5997.30	kN-mm
Direct Pressure N_{sd} / a	pa	19.29	N/mm ²
Permissible Stress $(0.67 \times f_{ck}) / 1.5$	fcd	20.10	N/mm ²
		OK	
Bending Stress Mt / Z	pb	0.99	N/mm ²
Permissible bending stress		14.85	N/mm ²
		OK	
Area on Pedestal	A_{c1}	490167.00	mm ²
$F_{rd} = A_{co} \times f_{cd} \times (\sqrt{A_{c1} / A_{c0}})$		4926.18	kN
$3 \times f_{cd} \times A_{co}$		7389.27	kN
$F_{rd} \leq 3 \times f_{cd} \times A_{co}$		OK	

Design Calculation of SLS- 1647 KN, ULS- 2365 KN Longitudinal Guided Bearing

Stress in top concrete

Top dispersion width			Dtpeff	375	
Thickness of Top Plate			Tt	22	
Area of Top Dispersion = $p \times d^{2/4}$	a		110491.071	mm ²	
Section Modulus = $p \times d^{3/32}$	Z		5179268.973	mm ³	
Vertical Load	Nsd	2365.00		kN	
Horizontal force	Vsd	0.00		kN	
Moment of resistance due to rotation:					
Rotation due to dead load	θ_p	0.00155		radians	
Live Load	θ_v	0.00225		radians	
Ratio = d_i / h_e		13.54			
For induced moment	k1	1.97			
	k2	76.38			
Induced moment due to rotation	Me.d				
$d_i^3 \times (k1 \times qp + k2 \times qv) / 1000$		5997.30		kN-mm	
Moment of resistance due to HF:					
Horizontal distance	C	162.50		mm	
Resultant HF	Vsd	0.00		kN	
Moment of resistance due to HF:	Mr.d	0.00		kN-mm	
$0.2 \times C \times Vsd$					
Total Movement = $Me.d + Mr.d =$	Mt	5997.30		kN-mm	
Direct Pressure Nsd / a	pa	21.40		N/mm ²	
Permissible Stress $(0.67 \times f_{ck}) / 1.5$	fcd	22.33		N/mm ²	
		OK			
Bending Stress Mt / Z	pb	1.16		N/mm ²	
Permissible bending stress		16.50		N/mm ²	
		OK			
Area on Superstructure	Ac1	441786.47		mm ²	
$F_{rd} = A_{co} \times f_{cd} \times (\sqrt{Ac1 / Ac0})$		4933.28		kN	
$3 \times f_{cd} \times A_{co}$		7399.92		kN	
$F_{rd} \leq 3 \times f_{cd} \times A_{co}$		OK			
Pot walls subjected to tensile force (Clause 5.3.1.2.3)					
$AR = (d_o - d_i) \times h_c$	AR	1800.00		mm ²	
$V_{c,sd} = 4 \times Nsd \times h_e / \pi \times d_i$	$V_{c,sd}$	222.48		kN	
$V_{fx,sd}$		0.00		kN	
$V_{sd} = V_{c,sd} + V_{fx,sd}$		222.48		kN	
$V_{rd} = f_y \times AR / Y_m$	$Y_m =$	556.36		kN	
	1.1	OK			
Pot walls subjected to shear force (Clause 5.3.1.2.4)					
$V_{sd} \leq V_{k,sd}$					
$V_{sd} = V_{c,sd} + 1.5 \times V_{fx,sd} / d_i$		0.68		kN	
$V_{rd} = (f_y \times (d_o - d_i)) / (2 \times Y_m \times (\sqrt{3}))$	$Y_m = 1.1$	5.15		kN	
		OK			
Pot base subjected to tensile force (Clause 5.3.1.2.5)					
$V_{sd} \leq V_{rd}$					
$A_p = d_o \times k_b$		7500.00		mm ²	
$V_{sd} = V_{c,sd} + V_{fx,sd}$		222.48		kN	
$V_{rd} = f_y \times A_p / Y_m$	$Y_m =$	2318.18		kN	
	1.1	OK			
Design resistance for integral guides					
Shear resistance					
Thickness of guide	ku	15.00		mm	
Height of guide	Hgb	18.00		mm	
Length of guide	Lgb	420.00		mm	
Length of side sliding	L	365.00		mm	
Height of application	ha	10.50		mm	
Effective length of guide	Leff	386.00		mm	
$V_{sd} \leq V_{rd}$	$Y_m =$				
$V_{rd} = k_u \times Leff \times f_y / \sqrt{3} \times Y_m$	1.1	1033.25		kN	
V_{sd}		0.00		kN	
		OK			

Design Calculation of SLS- 1647 KN, ULS- 2365 KN Longitudinal Guided Bearing

Bending resistance in combination with shear resistance

$P = ((2 \times V_{sd} / V_{Rd}) - 1)^2$	1.00	
$f^*y = (1 - P) \times f_y$	0.00	
$M_{Rd} = k_u \times 2 \times L_{eff} \times f^*y / (4 \times Y_m)$	0.00	kN-mm
$M_{sd} = V_{sd} \times h_a$	0.00	kN-mm
	OK	

Movement Capacity

Size of Top Plate	Lt	=	420.00	mm
	Bt	=	410.00	mm
Effective contact Concrete Diameter	Dtpeff	=	375.00	mm
Preset (If Any)		=	0.00	mm
Long. Movement Capacity (Lt - Dtpeff)		=	45.00	mm
Movement in One Direction		=	22.50	mm
Permissible Movement		=	21.00	mm
			OK	
Movement in Other Direction		=	22.50	mm
Permissible Movement		=	21.00	mm
			OK	

Anchor Bolts

Max Horizontal Force		0.00	kN
Min. Vertical Load		1150.00	kN
Frictional Force		0.00	kN
Diameter of Bolt		16.00	mm
Length of Bolt		35.00	mm
Number of Bolts	n	4.00	Nos.
Thickness of Collar	Ct	12.00	mm
Factor for Net Area	kn	0.78	
Effec. Area of Bolt $(p \times d274^2 \times kn / 4)$	Abolt	156.89	mm ²
σ_v		0.60	
f_{ub}		800.00	N/mm ²
Y_m		1.25	
f_u		570.00	N/mm ²
k_2		0.90	
Shear resistance $F_{v,Rd} = \sigma_v \times f_{ub} \times A / Y_m$	$F_{v,Rd}$	60.25	kN
Resultant horizontal force / bolt $V_{fy,sd} / n$	$F_{v,sd}$	0.00	kN
		OK	
Bearing resistance $F_{b,Rd} = 1.25 \times f_u \times d \times t / Y_m$	$F_{b,Rd}$	109.44	kN
Resultant horizontal force / bolt $V_{fy,sd} / n$		0.00	kN
		OK	
Tension resistance $F_{t,Rd} = k_2 \times f_{ub} \times A_s / Y_m$	$F_{t,Rd}$	90.37	kN
Design tension resistance	$F_{t,sd}$	0.00	kN
Combined shear and tension $= F_{v,sd} / F_{v,Rd} + F_{t,sd} / (1.4 \times F_{t,Rd})$		0.00	
		OK	

Anchor Sleeves (Clause - 5.3.6.4.2)

Top Plate			
Length of Sleeve		110.00	mm
Diameter of Sleeve		40.00	mm
Number of Sleeves		4.00	Nos.
Resistance offered by concrete $FR_{du} = 1.33 D \times L \times f_{cd} / \sqrt{3}$		75.46	kN
Design resistance in shear		0.00	kN
		OK	
Bottom Plate			
Length of Sleeve		120.00	mm
Diameter of Sleeve		40.00	mm
Number of Sleeves		4.00	Nos.
Resistance offered by concrete $FR_{du} = 1.33 D \times L \times f_{cd} / \sqrt{3}$		74.08	kN
Design resistance in shear		0.00	kN
		OK	

Design Calculation of SLS- 1647 KN, ULS- 2365 KN Longitudinal Guided Bearing

Requirement of clearance

$\delta = 0.01 \times D_i$	min. = 3	3.25	<	10	mm
$h_c - h_e - (w - w_e) \times 0.5 - (\theta \times 0.5 \times D_i)$		8.38			mm
δ		3.25			mm
		OK			
$h_p - (h_c - h_e) - (\theta \times 0.5 \times D_p)$		9.31			mm
δ		3.25			mm
		OK			

Curved Contact Surface (Clause - 5.3.1.4.2)

$V_{sd} \leq V_{rd}$	R	=	162.50	mm
	f_u	=	570.00	N/mm ²
	θ	=	0.004	Radian
	Y_m	=	1.10	
$V_{rd} = 15 \times f_u^2 \times R \times D_i / E_s \times Y_m^2$		=	1063.6	kN
V_{sd}		=	0.00	kN
		=	OK	
$w_e = 3.04 \times (\sqrt{1.5 \times V_{sd} \times R / E_s \times D_i})$		=	0.00	mm
$w_e + \theta \times D_i$		=	1.24	mm
Provided	w	=	6.00	mm
		=	OK	



BEARING TYPE & LOAD DETAILS		FIXED BEARING	LONG FIXED BEARING	TRANS FIXED BEARING	FREE BEARING	PIN BEARING	METALLIC GUIDED BEARING	
BEARING MARK		B1	B2	B3	B4	B5	B6	
GRADE OF CONCRETE	TOTAL QUANTITY (NO)		1	3	1	3	1	1
	UPPER SURFACE	SLS	M50	M50	M50	M50	M50	M50
		ULS	M50	M50	M50	M50	M50	M50
	LOWER SURFACE	ULS	M45	M45	M45	M45	M45	M45
DESIGN LOAD (KN)	SLS	MAX	1647	1647	1647	1647	-	-
		VERTICAL	843	843	843	843	-	-
		PERMENANT	673	673	673	673	-	-
		MIN	50	50	0	0	-	-
	ULS	LONGITUDINAL	0	0	0	0	-	-
		TRANSVERSE	2365	2365	2365	2365	-	-
		PERMENANT	1150	1150	1150	1150	-	-
		MIN	1	1	1	1	-	-
TRANSLATION (MM)	SLS	LONGITUDINAL	75	75	0	0	5548	0
		TRANSVERSE	0	0	0	0	2755	2755
		LONG	-	-	-	-	-	-
		IRREVERSABLE	-	-	-	-	-	-
	ULS	REVERSABLE	-	-	14	14	-	-
		TRAN	-	13	-	13	-	-
		LONG	-	-	-	-	-	-
		IRREVERSABLE	-	-	-	-	-	-
ROTATION (RED)	SLS	REVERSABLE	-	-	21	21	-	21
		TRAN	-	20	-	20	-	-
		LONG	0.00253	0.00253	0.00253	0.00253	-	-
		IRREVERSABLE	-	-	-	-	-	-
	ULS	REVERSABLE	0.00253	0.00253	0.00253	0.00253	-	-
		TRAN	-	-	-	-	-	-
		LONG	0.00380	0.00380	0.00380	0.00380	0.00380	0.00380
		IRREVERSABLE	-	-	-	-	-	-